



# Force distribution between interior and externally bonded reinforcement under cyclic loading Thorsten Leusmann

#### **Motivation**

Strengthening with externally bonded carbon fiber reinforced plastics (CFRP) is an effective technique to counteract aging and overloading of reinforced concrete bridges. For the prediction of the bridges service life the forces in the interior steel and the externally bonded reinforcement (EBR) have to be exactly known. But the different bond characteristics of EBR and interior reinforcement leads to a force distribution, which can be different from the distribution determined assuming a plane strain distribution. Experimental test results show that the strain in the EBR is higher than assumed particularly at low strain values, as they occur in the load cases of fatigue. In case of cyclic loading the difference between assumed and measured strain is reduced as a result of bond damage. However, an accurate prediction of the force distribution is only possible if the bond behavior of EBR and the cast-in reinforcement under cyclic loading is considered.

### **Model Aproach**

The test results can be recalculated taking the crack propagation rate da/dN for EBR and the bond coefficient d<sub>1</sub> for the consideration of the different bond characteristics into account using the following procedure for every load step: 1. Calculation of crack growth da

with upper, lower load and bond capacity  $F_1^0$ ,  $F_1^0$  and  $\Delta F_{IRd}$ 

## **Experimental Test**

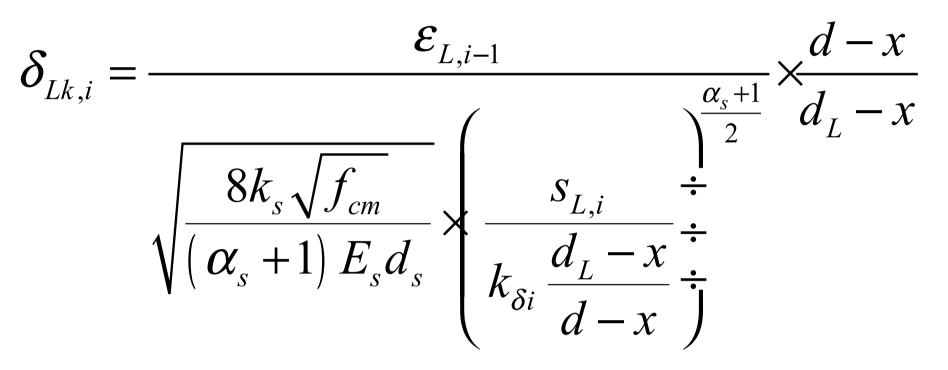
The test setup is designed as a combined double shear and pull-out test at two CFRP strips and four steel bars under cyclic loading. The cyclic tests are carried out in a hydraulic testing machine with pulsator for applying sinusoidal loads.

The concrete test specimen have a length I=1.35m and a square cross section with an edge length  $a_c=250$  mm. The two externally bonded CFRP strips have a bonded length  $I_1 = 1.15m$ , a width  $b_1 = 50mm$  and a thickness  $t_1 = 1.4$  mm and the four steel bars have a diameter  $d_s$  of 16 mm.

The CFRP strain is measured with a chain of strain gauges and the slip is measured with displacement transducers. The steel strains are measured with additional strain gauges on the steel bars inside the concrete in a bondless area

2. Determination of slip increase ds<sub>1</sub> and actual slip  $s_{11}$ 

3. Calculation of the actual bond coefficient  $\delta_{\mu}$ 



4. Calculation of the actual strain and force distribution

### Verification

The redistribution of the forces during the tests can be observed by the strain measurements and calculated with the model approach presented above. The decoupling caused by the crack formation in the concrete substrate along the bonded length leads to a decrease of the CFRP strain  $\varepsilon_{\rm l}$  and an increase of the steel strain  $\varepsilon_{c}$ .

